

**M.E. WATER RESOURCES AND HYDRAULIC ENGG. (EVENING)
SECOND YEAR SECOND SEMESTER - 2024**

**Groundwater Dynamics
(Paper - II)**

Time: Three hours

Full Marks: 100

**Answers any five questions.
Each question carries 20 marks.**

- 1 (a) Deduce the governing 1D equation for steady radial flow into a well for a confined, isotropic, and homogeneous aquifer.
- (b) A 610 mm diameter well penetrates vertically through a confined aquifer of 15 m thick. When the well is pumped $2800 \text{ m}^3/\text{d}$, the drawdown in a well 15 m away is 3 m and another well is 30 m away is 900 mm. What is approximate head in a pumped well for steady state conditions and what is the approximate drawdown in the well? Take the initial piezometric level as 30 m above the datum.
- [14+6=20]
- 2 (a) Deduce the governing 2D equation for unsteady flow of a homogeneous fluid in a, anisotropic, homogeneous media for unconfined aquifer.
- (b) A well fully penetrates a 25 m thick confined aquifer. After a long period of pumping at a constant rate of $0.06 \text{ m}^3/\text{s}$, the drawdowns at distances of 50 m and 150 m from the well were observed to be 3 m and 1.2 m respectively. What type of u consolidated deposit would you expect this to be?
- [14+6=20]
- 3 (a) Deduce the general flow equation for confined aquifer of an incompressible fluid considering isotropic, homogeneous and porous media.
- (b) During a constant permeability test on a soil sample of 100 mm diameter and 200 mm length, 0.004 m^3 of water was collected in 2.5 minutes under a constant of 0.5 m. Determine the permeability of the sample.
- (c) A soil has a coefficient of permeability 0.00082 m/s . If the kinematic viscosity is 0.009×10^{-4} , then calculate the intrinsic permeability of the sample.
- [10+5+5=20]

[Turn over

- 4 (a) Prove that equation which describes the transient drawdown in the piezometric head caused by a fully penetrating pumping well in a confined aquifer in unsteady state solution

$$s' = \frac{Q}{4\pi T} W(u)$$

Where, $W(u) = \int_u^\infty \frac{e^{-x}}{x} dx$ = well function; s' = drawdown (m); Q = well discharge (m^3/day); T = transmissivity (m^2/day).

- (b) A sand aquifer has a 10 percentile size of 0.50 mm and effective porosity of 0.35. If the temperature of the water in aquifer is 22°C , Estimate the range of linear velocity in case of Darcy's law is valid. Assume Kinematic viscosity at 22°C .

[15+5=20]

5. (a) "It is good practice to measure residual drawdowns" – justify the statement?

- (b) A well penetrating at a uniform rate of $2500 \text{ m}^3/\text{day}$ was shut down after 240 min, thereafter measurement of s and t tabulated in table were made in an observation well. Determine the transmissivity of the aquifer.

Table :Recovery test data (Pump shut down at $t=240$ min.)

| t (min) | s (m) |
|-----------|---------|
| 1 | 0.89 |
| 2 | 0.81 |
| 3 | 0.76 |
| 5 | 0.68 |
| 7 | 0.64 |
| 10 | 0.56 |
| 15 | 0.49 |
| 20 | 0.45 |
| 30 | 0.38 |
| 40 | 0.34 |
| 60 | 0.28 |
| 80 | 0.24 |
| 100 | 0.21 |
| 140 | 0.17 |
| 180 | 0.14 |

[4+16=20]

6. (a) Why Cooper Jacob straight line method is advantageous over the Theis method?
State its limitation.
- (b) In a test of a confined aquifer, the pumping rate was $500 \text{ m}^3/\text{day}$. Drawdown and time data were collected at an observation well 300m away (table given below) Determine the transmissivity and storativity of the aquifer using the Copper -Jacob Straight Line Method.

Table : Pumping test data

| Time (min) | Drawdowns (m) | Time (min) | Drawdowns (m) |
|------------|---------------|------------|---------------|
| 1.00 | 0.03 | 35.62 | 1.79 |
| 1.27 | 0.05 | 45.20 | 1.97 |
| 1.61 | 0.09 | 57.36 | 2.15 |
| 2.04 | 0.15 | 72.79 | 2.33 |
| 2.59 | 0.22 | 92.37 | 2.52 |
| 3.29 | 0.31 | 117.21 | 2.70 |
| 4.18 | 0.41 | 148.74 | 2.89 |
| 5.30 | 0.53 | 188.74 | 3.07 |
| 6.72 | 0.66 | 239.50 | 3.26 |
| 8.53 | 0.80 | 303.92 | 3.45 |
| 10.83 | 0.95 | 385.66 | 3.64 |
| 13.74 | 1.11 | 489.39 | 3.83 |
| 17.43 | 1.27 | 621.02 | 4.02 |
| 22.12 | 1.44 | 788.05 | 4.21 |
| 28.07 | 1.61 | 1000.00 | 4.39 |

[5+15=20]

7. (a) Briefly describe the approach and objective of groundwater modelling
- (b) An R.C network analog has to be constructed to simulate a confined aquifer of $40 \text{ km} \times 60 \text{ km}$ with an average thickness of 30 m, permeability of 25 m/day, and storage coefficient 4×10^{-4} . The maximum head is 40 m. The model can be represented by 40×60 nodes. Resistor of 3000Ω and capacitors of $0.01 \mu\text{F}$ are available; model voltage = 8 V. Work out the scale factors. If a calibrating resistor of 2500Ω is used for simulating pumping rate, determine the current pulse and excitation voltage to simulate a pumping rate $1000 \text{ m}^3/\text{day}$ at a particular node.

[5+15=20]

8. (a) A test was conducted with unconfined aquifer near river Damodar, Well was pumped at constant rate of $80 \text{ m}^3/\text{hr}$. The drawdown measured in an observation well 10 m away are listed in below table. The aquifer thickness is 20 m. Calculate the hydraulic parameter for aquifer using Numan's straight-line method

Ex/PG/DB/SWRE/11/2024

| TIME SCIENCE PUMPING BEGAN (min) | S DRAWDOWN (m) |
|--|----------------------|
| 1 | 0.01 |
| 4 | 0.01 |
| 5 | 0.01 |
| 6 | 0.01 |
| 7 | 0.02 |
| 8 | 0.02 |
| 9 | 0.02 |
| 10 | 0.03 |
| 11 | 0.03 |
| 12 | 0.03 |
| 13 | 0.04 |
| 14 | 0.05 |
| 53 | 0.06 |
| 55 | 0.06 |
| 63 | 0.07 |
| 69 | 0.08 |
| 88 | 0.09 |
| 91 | 0.10 |
| 94 | 0.10 |
| 97 | 0.10 |
| 102 | 0.11 |
| 107 | 0.11 |
| 112 | 0.12 |
| 237 | 0.23 |
| 357 | 0.29 |
| 417 | 0.30 |
| 537 | 0.31 |
| 657 | 0.31 |
| 777 | 0.32 |
| 897 | 0.32 |
| 1282 | 0.33 |
| 1497 | 0.33 |