Ref No. EX/PG/CE/T/128C/2017

## MASTER OF CIVIL ENGINEERING 2nd SEMESTER. 2017

## (2<sup>nd</sup>Semester) SUBJECT: RETAINING STRUCTURES AND UNDERGROUND CONSTRUCTION Time: Three hour Full Marks: 100

1. Answer ALL questions.

Use a separate Answer-Script for each part

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No. of Question	<ul> <li>2. Assume reasonable values of data if it is not supplied.</li> <li>3. No code etc. will be needed to answer the questions of this part</li> <li>4. Notations used bear their usual meanings Part –I (40 Marks)</li> </ul>	<b>M</b> arks
Q.1 a) b)	Illustrate Rowe's moment reduction technique for anchored sheet pile walls.  A cantilever sheet pile wall is required to be provided at a site. The relevant data are as follows:  Dredge line: 8m from GL	5
Q.2	Water Table : 2m below GL Subsoil consists of clayey sand down to dredge line. Properties of clayey sand : $\gamma$ =19.5 kN/m³, c= 25 kN/m² and $\Phi$ = 20° Soil below dredge line : purely cohesive : $\gamma$ =19.0 kN/m³, c= 38 kN/m² and $\Phi$ = 0 Determine the depth of embedment to be provided. A retaining wall, 6m high, supports a backfill ( $\gamma$ =20 kN/m³, c= 0 kN/m² and $\Phi$ = 30°). If the wall is pushed towards the backfill, a)Compute the total active earth pressure on the wall and also find its point of application	10
Q.3	b) Calculate the factor of safety against overturning and sliding if the details of cross section of the retaining wall are as follows:  Top width=0.5m, depth below.ground = 2m, width of wall at its junction with base = 0.7m, projection towards heel= 2.5m and that towards toe=0.6m. The wall is vertical on the side of heel and tapered on the other side. Thickness of base=0.7m.  Illustrate how stability of a diaphragm type cellular cofferdam is checked.	15 10

MASTER OF ENGINEERING IN CIVIL ENGINEERING EXAMINATION, 2017

(1<sup>ST</sup> YEAR 2<sup>ND</sup> SEMESTER)

(4<sup>st</sup>/2<sup>nd</sup> Semester/Repeat/Supplementary/Spl. Supplementary/Old/-Annual/-Biannual)
SUBJECT: RETAING STRUCTURES AND UNDERGROUND CONSTRUCTION

(Name in full)

Full Marks 30/100

Time: Two hours/Three hours/Four hours/ Six hours

No. of Questions	PART II (MARKS 60)	Marks
	Answer all questions	
Q1	For construction of an underground shopping complex cum car parking it is proposed to do the excavation by using 800 mm thick RCC braced walls.  Size of excavation: 75 m x 25 m  Depth of excavation: 12 m  Depth of braced wall: 20 m	10+10+10
	Subsoil data at the proposed site are as given below: Depth $0-4.0m$ : Brownish grey silty clay / clayey silt $ \gamma = 18.6 \text{ kN/m}^3 \text{ , } C_u = 40 \text{ kN/m}^2 \text{, } m_v = 0.0004 \text{ m}^2 \text{/kN} $ Depth $4.0-15.0m$ : Dark grey silty clay / clayey silt with decomposed wood $ \gamma = 17.5 \text{ kN/m}^3 \text{ , } C_u = 25 \text{ kN/m}^2 \text{, } m_v = 0.0006 \text{ m}^2 \text{/kN} $ Depth $15.0-18.0m$ : Stiff / very stiff bluish / mottled brown silty clay / clayey silt with rusty spots	
	$\gamma = 19.5 \text{ kN/m}^3, C_u = 75 \text{ kN/m}^2, m_v = 0.0003 \text{ m}^2/\text{kN}$ Depth > 18.0m till 35.0m : Dense / very dense sand $\gamma = 20 \text{ kN/m}^3, \phi = 36^0$ Ground water table at a depth of 1.0m below G.L.	
	<ul> <li>(i) Check the stability of braced excavation</li> <li>(ii) Draw the earth diagram may develop on the soil side of the wall</li> <li>(iii) Determine moments and forces on the walls, wales and struts</li> </ul>	
Q2	Discuss the term 'ground loss' with respect to tunneling through clayey soil highlighting how it governs the settlement trough at the ground level.  Draw the settlement trough of the ground surface above a typical 5.5m diameter tunnel with depth of the axis 11.0m below ground level. Take maximum settlement at the ground surface = 50 mm.  Subsoil at the site is same as that given in Q1.	15
Q3	A tunnel of diameter 1.5 m is to be constructed at a depth of 5.0m below ground level (ground level to axis level). Expected ground loss = 5% of the face area. Calculate the bending strain which will develop during tunneling in a transverse pipeline of outside diameter 500 mm and wall thickness 18 mm lying above the tunnel at a depth of 1.5 m below ground level.  Pipe secant modulus = $7 \times 10^{10} \text{ N/mm}^2$ Soil secant modulus = $10 \times 10^{10} \text{ N/mm}^2$ Soil poisson's ratio = 0.5	15